

DECLARAȚIA DE PERFORMANȚĂ

DoP Nr.: MKT-531 - ro

♦ Cod unic de identificare al produsului-tip: Sistem de injectare VME

Utilizare (utilizări) preconizată (preconizate): Conexiune ulterioară cu armătură de legătură cu sistemul

de injecție VME, a se vedea anexa / Annex B

♦ Fabricant: MKT Metall-Kunststoff-Technik GmbH & Co.KG

Auf dem Immel 2 67685 Weilerbach

♦ Sistemul (sistemele) de evaluare şi de verificare a constanței performanței:

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♦ Documentul de evaluare european:

EAD 330087-00-0601

Evaluarea tehnică europeană:

ETA-07/0299, 09.11.2017

Organismul de evaluare tehnică:

DIBt, Berlin

Organism (organisme) notificat(e):

NB 1343 – MPA, Darmstadt

→ Performanţa (performanţe) declarată (declarate):

Caracteristici esenţiale	Performanţă
Rezistență mecanică și stabilitate (BWR1)	
Factor de amplificare α _{lb} , Compozit voltaj f _{bd} Anexa/Annex C1	
Securitatea la incendiu (BWR2)	
Comportamentul la foc	Clasa A1
Rezistență la foc	Anexa/Annex C2 – C3

Performanța produsului identificat mai sus este în conformitate cu setul de performanțe declarate. Această declarație de performanță este eliberată în conformitate cu Regulamentul (UE) nr. 305/2011, pe răspunderea exclusivă a fabricantului identificat mai sus.

Semnată pentru și în numele fabricantului de către:

Compozit voltaj

Stefan Weustenhagen

(Director general)
Weilerbach, 09.11.2017

Dipl.-Ing. Detlef Bigalke

(Sef de dezvoltare a produselor)



Originalul acestei declarații de performanță a fost scris în limba germană. În cazul abaterilor în traducere, versiunea germană este validă.

Specifications of intended use

Anchorages subject to:

- Static or quasi static action
- Fire exposure

Base material:

- Reinforced or unreinforced normal weight concrete acc. to EN 206-1:2000.
- Strength classes C12/15 to C50/60 acc. to EN 206-1:2000.
- Maximum chloride concrete of 0,40 % (CL 0,40) related to the cement content acc. to EN 206-1:2000.
- Non-carbonated concrete.

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of \emptyset + 60 mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Temperature range:

- 40 °C to +80 °C (max. long term temperature +80 °C and max. short term temperature +50 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc plated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure including industrial and marine environment or exposure to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where deicing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position
 of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to
 supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages are designed in accordance with EN 1992-1-1 and Annex B2.
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

Installation:

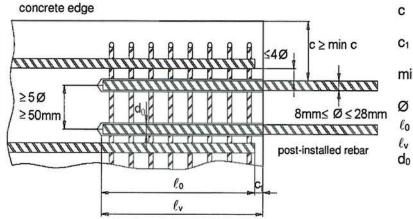
- Dry or wet concrete
- The rebar connection must not be placed in water-filled boreholes.
- Overhead installation admissible
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint)
- Hole drilling by hammer drill, vacuum drill, compressed air drill or diamond drill mode.
- While curing of the injection mortar, the temperature of the member shall not fall below +5°C and must not exceed +40°C.
- The installation of post-installed rebar or tension anchor ZA shall be done only by suitable trained installer and
 under supervision on site; the conditions under which an installer may be considered as suitable trained and the
 conditions for supervision on site are up to the member states in which the installation is done.

Injection System VME	
Intended use Specifications of intended use	Annex B1

General construction rules for post-installed reinforcing bars and tension anchor ZA

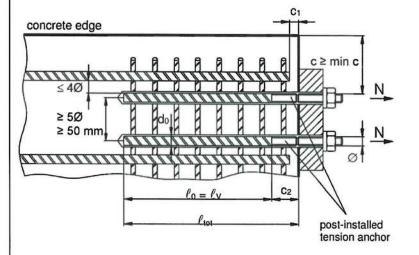
- Bond strength f_{bd} acc. to EN 1992-1-1 may be applied.
- The shear transfer between old and new concrete shall be designed acc. to EN 1992-1-1.
- If the clear distance of overlapping bars is greater than 4 Ø, the lap length must be increased by a length equal to the clear space where it exceeds 4 Ø.
- The minimum concrete cover acc. to EN 1992-1-1 shall be kept.

Figure B1: Post-installed rebars



- concrete cover of post-installed
 - reinforcement
- c₁ concrete cover at front end of cast
 - in-place rebar
- min c minimum concrete cover according
 - Table B1
- Ø diameter of post-installed rebar
- ℓ_0 lap length acc. to EN 1992-1-1
 - embedment depth $\ell_{\rm v} \ge \ell_0 + c_1$
- do nominal drill bit diameter according to
 - Annex B4, Table B4

Figure B2: Tension anchor ZA



- c concrete cover of post-installed
 - tension anchor ZA
- c₁ concrete cover at front side of
- cast-in-place rebar
- min c minimum concrete cover
 - acc. to Table B1
- c₂ length of bonded thread
 - $(c_2 > c_1; see Table B1)$
- Ø diameter of tension anchor
- ℓ_0 lap length acc. to EN 1992-1-1
- ℓ_{tot} embedment depth $\ell_{tot} \ge \ell_0 + c_2$
- do nominal drill bit diameter according
 - Annex B4, Table B4

Injection System VME

Intended use

General construction rules

Table B1: Minimum concrete cover min c1) of post-installed rebars and tension anchor ZA depending on drilling method and drilling tolerances

Drilling method	Rebar diameter	min c (without drilling aid device)	min c (with drilling aid device)
Hammer drilling, vacuum drilling,	< 25 mm	30 mm + 0,06 ℓ _v ≥ 2 Ø	30 mm + 0,02 ℓ _v ≥ 2 Ø
diamond core drilling	≥ 25 mm	40 mm + 0,06 ℓ _v ≥ 2 Ø	40 mm + 0,02 ℓ _v ≥ 2 Ø
Compressed air	< 25 mm	50 mm + 0,08 ℓ _v	50 mm + 0,02 ℓ _v
drilling	≥ 25 mm	60 mm + 0,08 ℓ _v	60 mm + 0,02 ℓ _v

¹⁾ Minimum concrete cover acc. to EN 1992-1-1 must be observed.

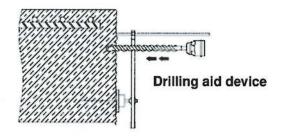


Table B2: Dimensions and installation parameters of tension anchor ZA

Anchor size				M12	M16	M20	M24
Rebar diameter		Ø	[mm]	12	16	20	25
Cross-section area (th	readed part)	As	[mm²]	84	157	245	353
Width across nut flats		SW	[mm]	19	24	30	36
Effective embedment	depth	ℓ_{v}	[mm]	and the second	$\ell_{\mathbf{v}} = \ell$	tot - C2	
Length of bonded thread steel, zinc plated		C ₂	[mm]	≥ 20	≥ 20	≥ 20	≥ 20
uneau	A4 / HCR			≥ 100	≥ 100	≥ 100	≥ 100
Maximum installation	torque	Tinst	[Nm]	50	100	150	150

Table B3: Maximum embedment depth

Rebar / Tension Anchor ZA	Motar temperature	Max. embedment depth $\ell_{ m v,max}$
Ø 8 to 12 mm	+5°C to +19°C	130 cm
Ø 6 10 12 mm	≥20°C	200 cm
Ø 14 to 28 mm	+5°C to +19°C	200 cm
Ø 14 to 26 mm	≥20° C	280 cm

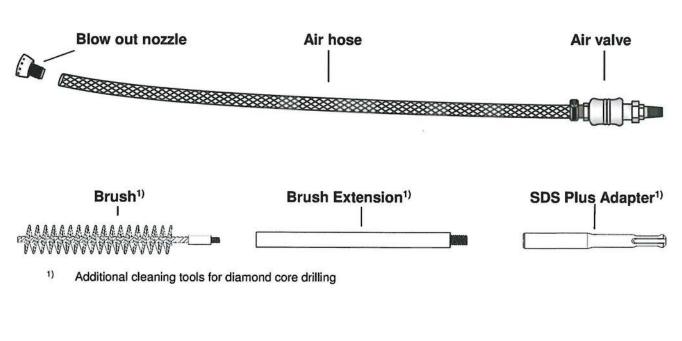
Injection	System	VME
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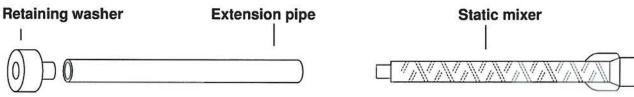
Intended use

Minimum concrete cover, installation parameters, maximum embedment depth

Table B4: Cleaning and installation tools

Rebar Ø	Tension anchor ZA	Drill bit diameter d₀	Blow out nozzle Ø	Brush Ø (diamond drilling)	Extension pipe (outer Ø)	Retaining washer Ø
[mm]	[-]	[mm]	[mm]	[mm]	[mm]	[mm]
8	-	12	10	13,0		10
10	-	14	10	15,5	10	13
12	M12	16	14	17,5		15
14	-	18	14	19,5		17
16	M16	20	17	22		19
20	M20	25	17	27		24
24	-	30	27	32	16	29
25	M24	32	27	34		31
26	-	32	27	34		31
28	- 1	35	27	37		34





Injection System VME	
Intended use Cleaning and Installation tools	Annex B4

Table B5: Working time and curing time

Temperature in the bore hole	Working time	Minimum curing time
+5°C to +9°C	60 min	72 h
+10°C to +19°C	45 min	36 h
+20°C to +29°C	30 min	10 h
+30°C to +39°C	20 min	6 h
+40°C	12 min	4 h

Note: A mortar temperature > + 20 ° C reduces the press out forces and accelerates the injection of the mortar. When using the adhesive in wet concrete the curing times have to be doubled.

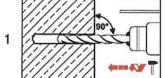
Installation instructions

Preperation:

- Remove carbonated concrete and roughen concrete surface prior to installation of post-installed rebars.
- b. Determine position and diameter of existing reinforcement. Only the admissible force of the existing reinforcement can be transmitted into the post-installed rebar connection
- c. The drilling aid device must be adjusted towards the existing reinforcement
- d. The concreting joints must be roughened to at least such an extent that the aggregates protrude

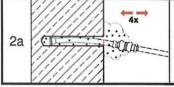
Making and cleaning of hammer drilled, vacuum drilled and compressed air drilled holes

Bore hole drilling



Select drill bit diameter according to Table B4. Build drill hole by hammer drilling, vacuum drilling or compressed air drilling. In case of aborted holes, the bore holes must be filled with mortar.

Cleaning



Assemble provided blow out nozzles, air hoses and air valve and connect to compressed air (≥ 6 bar). Place marking on the air hose at a distance from the end equal to the drill hole depth. Open compressed air valve and blow out 4x from top to bottom and reverse. The air hose marking has to get below the concrete surface in order to ensure cleaning to the very bottom of the hole.

Injection System VME

Intended use

Working and curing time, Installation instructions – Preperation; Making and cleaning of hammer drilled, vacuum drilled and compressed air drilled holes

Installation instruction (continuation) Marking and cleaning of diamond core drilled holes Bore hole drilling 1a Drill hole by using a diamond drill with diamond drill bit. 1b Remove drill core at least up to the nominal hole depth and check drill hole depth. Cleaning Flush drill hole with water, staring from the bottom, until clear water gets out of the 2a drill hole. Choose brush suitable to the hole. Brush bore hole 2x using a cordless screwdriver 2b or a drilling machine. Use brush extension for deep holes. Flush drill hole again with water, staring from the bottom, until clear water gets out of 2c the drill hole. Assemble provided blow out nozzles, air hoses and air valve and connect to compressed air (≥ 6 bar). Place marking on the air hose at a distance from the end 2d equal to the drill hole depth. Open compressed air valve and blow out 2x from top to bottom and reverse. The air hose marking has to get below the concrete surface in order to ensure cleaning to the very bottom of the hole. 2e Brush the hole again 2x (according to step 2b). 2f Blow out again 2x (according to step 2d).

Injection System VME

Intended use

Installation instructions

Making and cleaning of diamond core drilled holes

Installation Instruction (continuation)

Inje	ection into bore hole	
За	ev annonnonnon	Mark the position of the embedment depth ℓ_{ν} on the rebar.
3b	- W	Check bore hole depth by inserting rebar or tension anchor up to the marking in the empty hole.
4	The state of the s	Prepare cartridge with static mixer, extension pipe and retaining washer. Extension pipe must correspond the hole depth.
5		Attach marking length ℓ_m on extension pipe for injection of adhesive: a) Estimation: $\ell_m = 1/3 * h_1 [mm]$ b) Precise formula for optimum volume (compressed air drilling): $\ell_m = h_1 * (1,2 * \frac{\emptyset^2}{d_0^2} - 0,2) [mm]$ $\ell_m \text{Length from the end of the retailing washer to the mark on the mixer extension}$ $h_1 \text{drill hole depth} = \text{embedment depth} \ (\ell_v \text{resp.} \ \ell_{\text{tot}})$ $\emptyset \text{rebar diameter}$ $d_0 \text{nominal drill bit diameter}$ When using hammer drill or diamond core, the length ℓ_m may be multiplied by 1,10.
6	mln. 10cm	Prior to applying, discard at least a 10 cm line of mortar (forerun) until the mortar shows a consistent colour. Never use this mortar!
7		Fill the hole starting from the bottom with injection mortar VME without air bubbles. Inject until the marking ℓ_m on the extension pipe can be seen.

Injection System VME

Installation Instruction (continuation)

Push the rebar or tension anchor into the hole while turning slightly until the embedment depth is reached. The bar should be free of dirt, grease, oil or other foreign material. Excessive adhesive must exceed out of the drill hole. Otherwise pull out rebar, drill out hole after curing of the adhesive and start again from No. 2. Keep curing time according to Annex B5, Table B5. Do not move or load the rod during curing time. After curing, the rod can be loaded.

Injection	System	VME
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Installation instructions
Installation of rebar or tension anchor

Minimum anchorage length and minimum lap length

The minimum anchorage length $\ell_{b,min}$ and the minimum lap length $\ell_{0,min}$ according to EN 1992-1-1 ($\ell_{b,min}$ acc. to Eq. 8.6 and Eq. 8.7 and $\ell_{0,min}$ acc. to Eq. 8.11) shall be multiplied by the amplification factor α_{lb} acc. to Table C1.

Table C1: Amplification factor alb

Concrete strength class	Drilling method	Amplification factor α_{lb}
C12/15 to C50/60	Hammer drilling, vacuum drilling or compressed air drilling	1,0
G12/15 to C50/60	Diamond coring	1,5

Table C2: Design values of the ultimate bond resistance fbd1)

Rebar Tension Anchor ZA		Concrete strength class								
		C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Design value of bond resistance fbd										
Hammer drilling, vacuum drilling or compressed air drilling	[N/mm²]	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3
Diamond coring	[N/mm²]	1,6	2,0	2,3	2,7	3,0	3,0	3,4	3,7	3,7

With η_1 =1,0 acc. to EN 1992-1-1 for good bond conditions (for all other bond conditions multiply the values by 0,7)

Injection System VME	
Performances	Annex C1
Amplification factor	7
Design values of ultimate bond resistance fbd	

Design value of ultimate bond stress fbd,fi under fire exposure for concrete classes C12/15 to C50/60 (all drilling methods):

The design value of ultimate bond stress fbd,fi under fire exposure will be calculated by the following equation:

$$f_{bd,fi} = k_{b,fi}(\theta) \cdot f_{bd} \cdot \gamma_c / \gamma_{M,fi}$$

with: $\theta \le 270^{\circ}\text{C}$ $k_{\text{b,fi}}(\theta) = 9221,2 * \theta^{-1,747} / (f_{\text{bd}} * 4,3) \le 1,0$

 $\theta > 270$ °C $k_{b,fi}(\theta) = 0$

f_{bd,fi} design value of ultimate bond stress in case of fire in N/mm²

 θ Temperature in °C in the mortar layer $k_{b,fi}(\theta)$ Reduction factor under fire exposure

f_{bd} Design value of the ultimate bond stress in N/mm² in cold condition according to

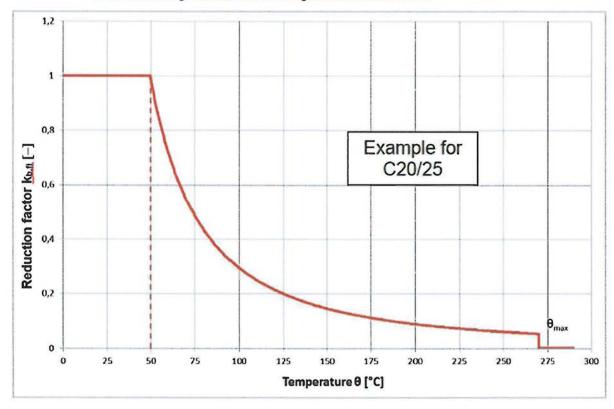
Table C2 considering concrete class, rebar diameter, drilling method and the bond

conditions according to EN 1992-1-1.

 γ_c partial factor acc. to EN 1992-1-1 $\gamma_{M,fi}$ partial factor acc. to EN 1992-1-2

For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent ultimate bond stress fbd.fi.

Picture C1: Example graph of reduction factor k_{b,fi}(θ)
Concrete strength class C20/25 for good bond conditions





Design value of ultimate bond stress fbd,fi under fire exposure

Annex C2

Table C3: Characteristic tension strength in case of fire for tension anchor ZA, concrete strength class C12/15 to C50/60, acc. to Technical Report TR 020

Tension anchor ZA				M12	M16	M20	M24	
Steel, zinc plated								
Characteristic tension strength	R30			20				
	R60		[N/mm2]	15				
	R90	O Rk,s,fi	[N/mm²]		1	3		
	R120				1	0		
Stainless steel A	4, HCR		- 10					
Characteristic tension strength	R30	05			3	0		
	R60		[N/mm2]		2	5		
	R90	O Rk,s,fi	[N/mm²]		2	0		
	R120				1	6		

Design value of the tension strength $\sigma_{\text{Rd,s,fl}}$ under fire exposure for tension anchor ZA

The design value of the steel strength $\sigma_{Rd,s,fi}$ under fire exposure will be calculated by the following equation:

$$\sigma_{Rd,s,fi} = \sigma_{Rk,s,fi} / \gamma_{M,fi}$$

with:

 $\sigma_{\text{Rk,s,fi}}$

characteristic steel strength acc. to Table C3

γ_{M,fi} partial factor under fire exposure acc. to EN 1992-1-2

Injection	System	VME
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